

# **BS EN 1994-1-1 for composite structures of steel and concrete**

Roger Johnson

University of Warwick

# The ten Eurocodes

|                |                                   |                 |                |
|----------------|-----------------------------------|-----------------|----------------|
| <b>EN 1990</b> | <b>Basis of design</b>            | <b>1 Part</b>   | <b>2000</b>    |
| <b>EN 1991</b> | <b>Actions</b>                    | <b>10 Parts</b> | <b>2000-03</b> |
| <b>EN 1992</b> | <b>Concrete structures</b>        | <b>4 Parts</b>  | <b>2002-04</b> |
| <b>EN 1993</b> | <b>Steel structures</b>           | <b>20 Parts</b> | <b>2002-04</b> |
| <b>EN 1994</b> | <b>Composite structures</b>       | <b>4 Parts</b>  | <b>2002-04</b> |
| <b>EN 1995</b> | <b>Timber structures</b>          | <b>3 Parts</b>  | <b>2001-03</b> |
| <b>EN 1996</b> | <b>Masonry structures</b>         | <b>5 Parts</b>  | <b>2002-03</b> |
| <b>EN 1997</b> | <b>Geotechnical design</b>        | <b>3 Parts</b>  | <b>2002-03</b> |
| <b>EN 1998</b> | <b>Seismic design</b>             | <b>6 Parts</b>  | <b>2002-03</b> |
| <b>EN 1999</b> | <b>Aluminium alloy structures</b> | <b>3 Parts</b>  | <b>2003</b>    |

# Eurocodes BS EN 1990 to 1999

To remove barriers to trade in construction works in the European Union

To harmonise design philosophy and methods:

- **across structural materials** (steel, concrete, masonry, timber, geotechnical, etc.)
- **across types of structure** (buildings, bridges, masts, chimneys, pipelines, foundations, etc)
- **across Europe**, from Iceland to Greece, and Latvia to Portugal

# The scope of BS EN 1994-1-1, 'Composite structures of steel and concrete' includes:

- **steels** up to grade S460
- **concretes** up to grades C60/75 and LC60/66
- unbraced and braced **frames**, with simple, semi-rigid or rigid **joints**
- concrete-encased **columns** and concrete-filled tubes
- partially-encased **beams**

# Scope of EN 1994 (continued)

- **box girders** and composite plates
- **trusses** with either or both chords composite
- bowstring arches and half-through bridges with the **deck in axial tension**
- **prestress** by external tendons or by jacking at supports
- resistance of buildings to **fire**, by three alternative methods

# EN 1994-1-1 compared with BS 5950-3-1

In BS 5950, not in EC4:

- transverse reinforcement above beams (in EC2)
- slenderness limits for Classes 1 to 3 (in EC3)

Not in BS 5950 but in EC4:

- Effective width of concrete flanges
- Control of width of cracks in concrete
- Lateral torsional buckling of beams

# Design compressive strength of concrete in flexure and compression

In EN 1992-1-1:  $f_{cd} = \alpha_{cc} f_{ck} / \gamma_C$

in its NA:  $\alpha_{cc} = 0.85$

In EN 1994-1-1:  $f_{cd} = f_{ck} / \gamma_C$

and factor 0.85 appears in the formulae for resistances of beams and columns

# Modular ratios

(with simplifications for buildings)

|                               | Variable actions | Permanent actions<br>(for creep factors from 1 to 3) |
|-------------------------------|------------------|--|
| BS 5950-3-1                   | 6                | 18   |
| EC4, C20/25<br>for shrinkage: | 7<br>-           | 15 to 30 (14)<br>12 to 18 (omit)                     |
| EC4, C60/75<br>for shrinkage: | 5.5<br>-         | 11 to 23 (11)<br>8 to 14 (omit)                      |



# Simplifications permitted for buildings

## Conditions:

- No members susceptible to: fatigue  
sidesway instability  
lateral-torsional buckling

## Then:

In global analyses for Ultimate Limit States neglect:

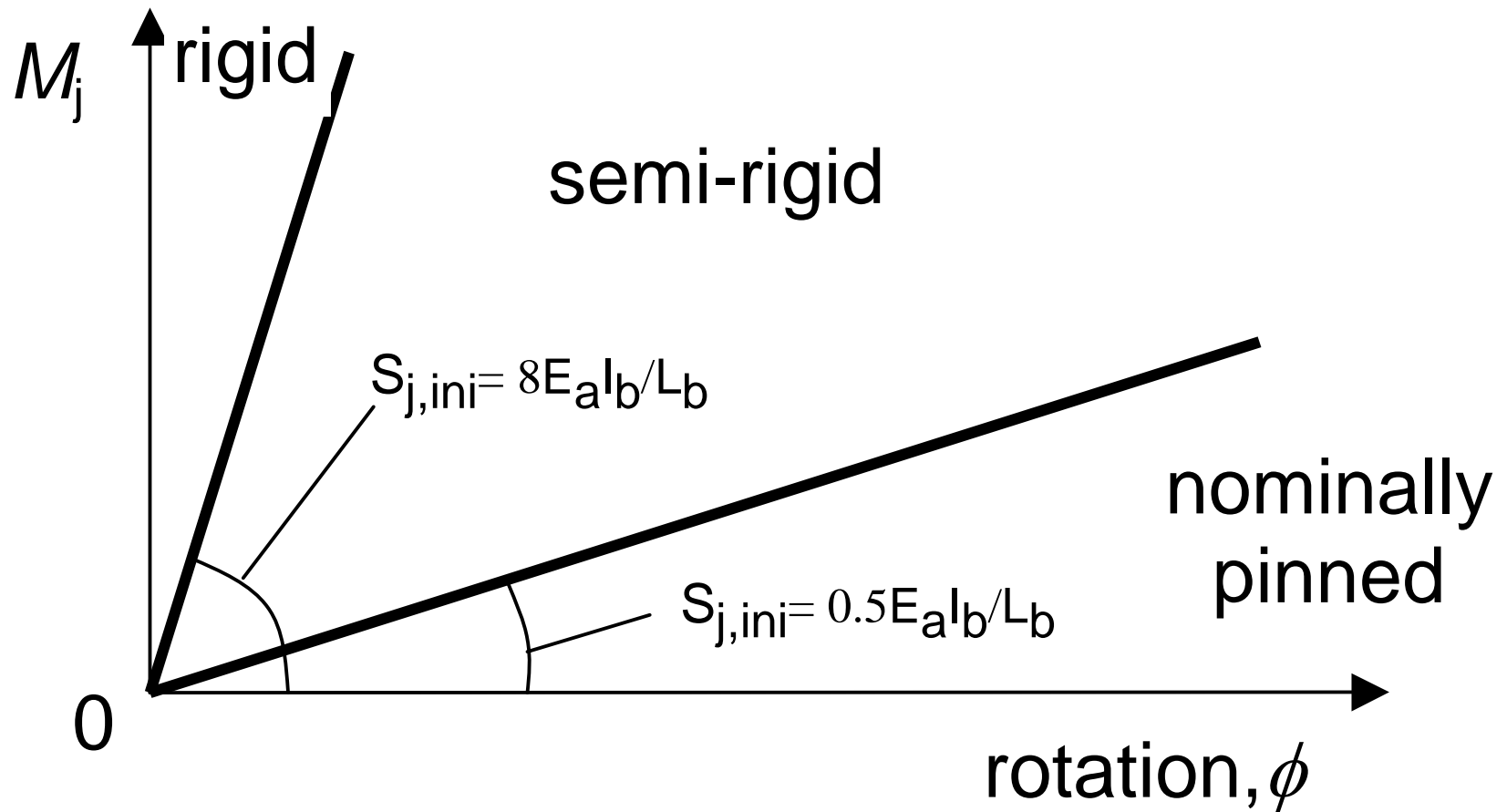
- primary and secondary shrinkage and creep
- all effects of staged (unpropped) construction
- differential temperature and settlement

# Bending moments in members of a no-sway frame

‘Simple’ design, avoiding continuous beams

- Choose ‘simple’ beam-to-column joints
- Check that Class is ‘nominally pinned’, to EC3-1-8 or from ‘experience’ (cl. 8.3.4(2))
- Design beams as simply-supported, with reinforcement at supports to cl. 7.4.1(4), to avoid wide cracks
- Decide location of ‘nominal pins’; hence BMs in columns are known

# Classification of joints by initial stiffness



# Bending moments in members of a no-sway frame

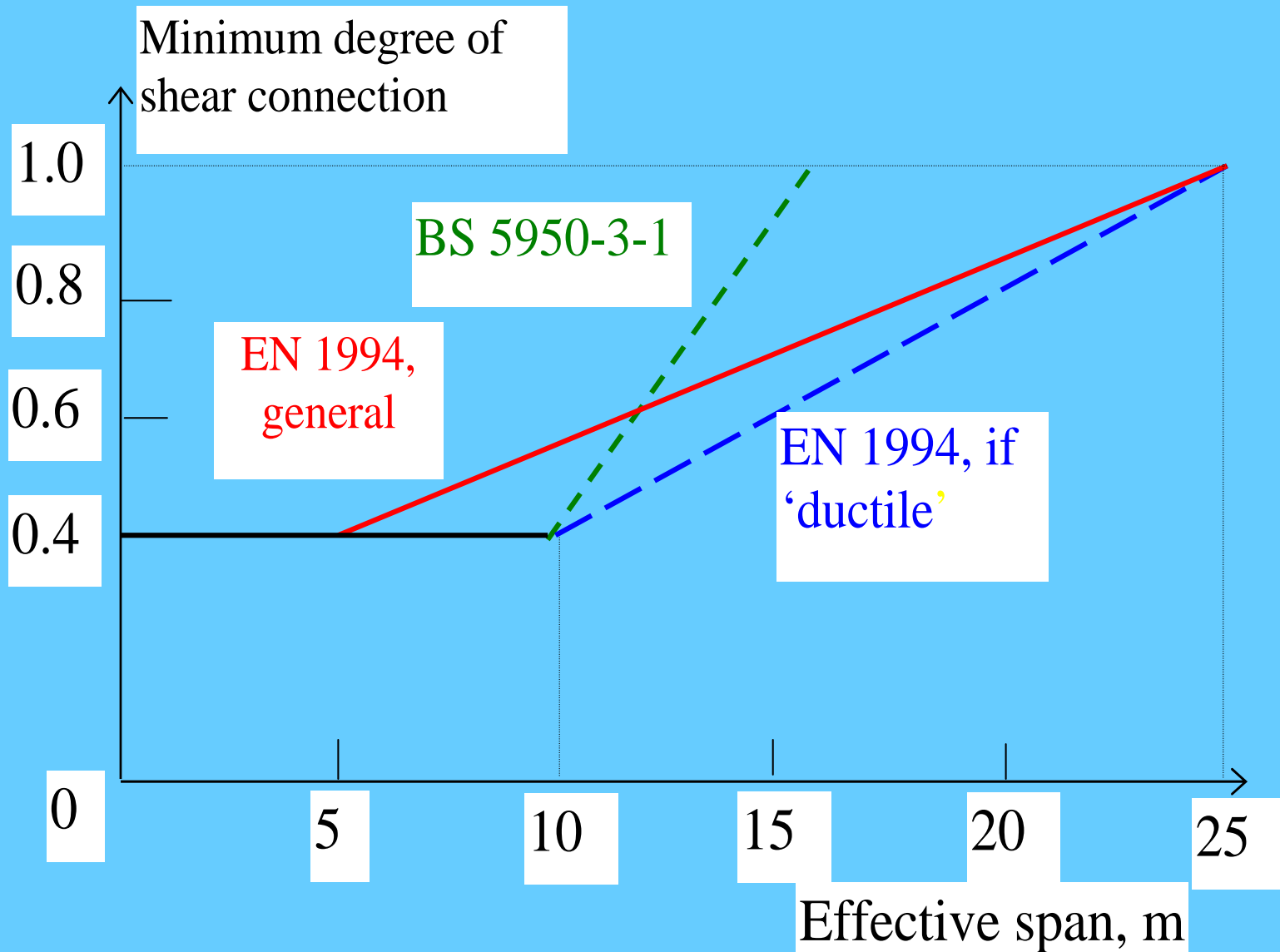
## Design with continuous beams

- Choose 'rigid' beam-to-column joints to EC3-1-8 and EC4
- Do elastic global analysis, with 'uncracked' cross-sections
- Redistribute moments (RoM) in beams by up to 20% (Cl.3) to 40% (Cl.1)
- No RoM permitted in composite columns

[Note: include effects of imperfections in checks on members]

**OR:** use semi-rigid joints for efficient structure but more work

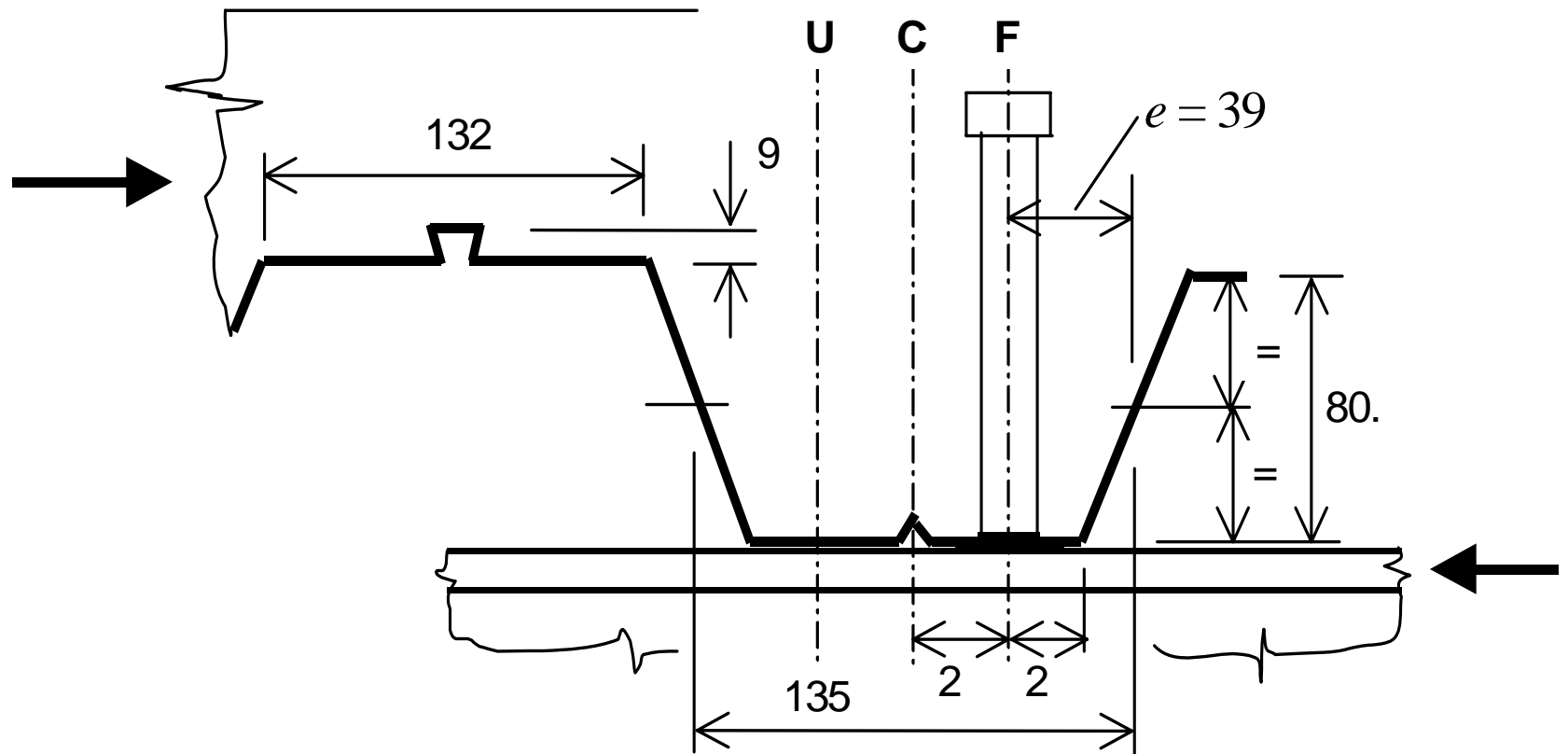
# Partial shear connection in composite beams



# Studs in troughs of sheeting

- Slip capacity needed for uniform spacing
- 6 mm sufficient in solid slabs
- Studs work well in solid slabs
- Some slip capacities in sheeting  $< 2$  mm
- Caution where two or more of
  - studs off-centre in troughs
  - two or more studs per trough
  - heavy point loads
  - long spans
  - low degree of shear connection
  - edge beam

# Trough in sheeting with a stud in the 'F' location

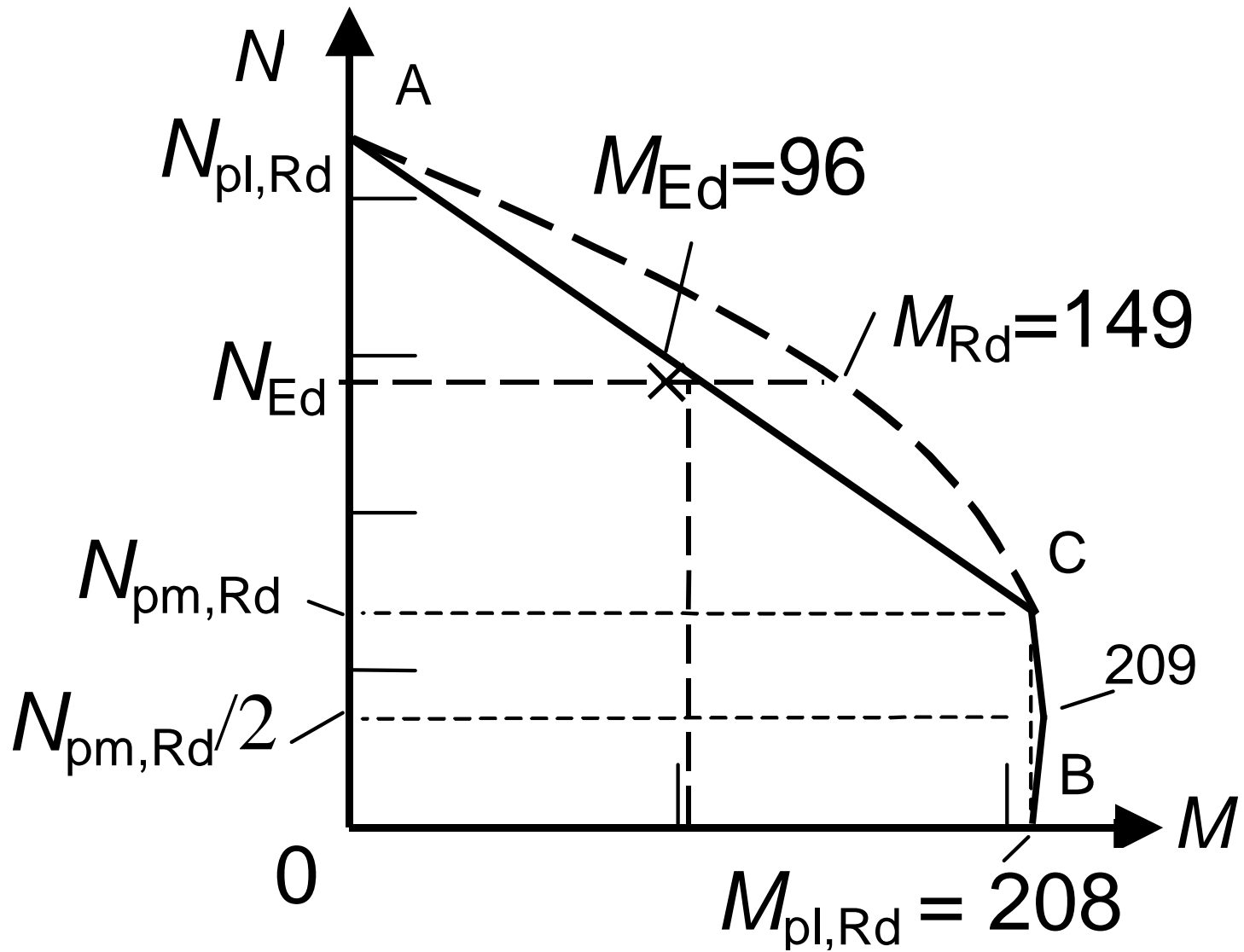


# Composite columns in EN 1994-1-1

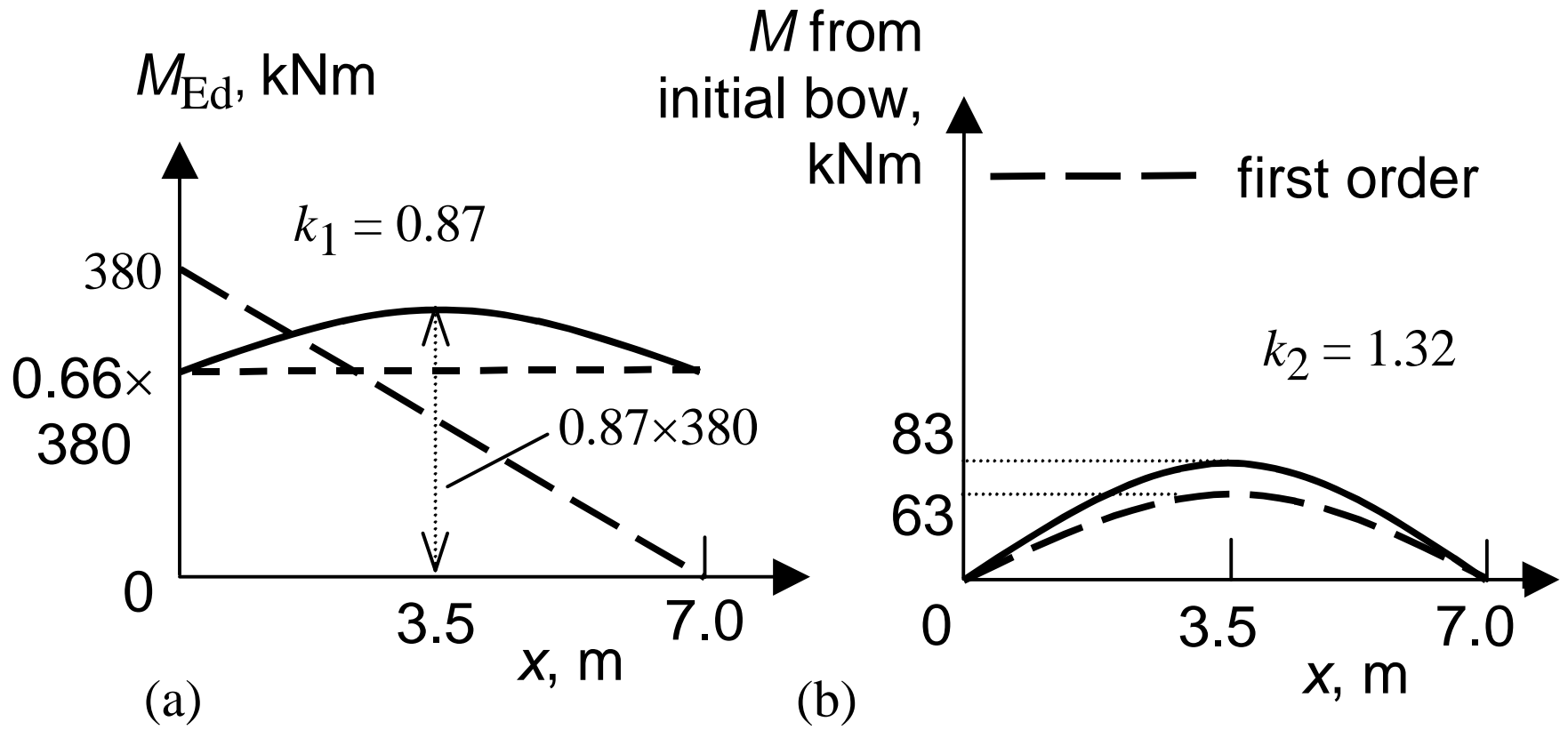
- Scope: web-encased steel sections  
fully-encased steel sections  
concrete-filled steel tubes
- Simple method for uniform doubly symmetrical sections
- Back to basics: different from rules for columns in EC2 and EC3
- No use of effective lengths or column curves
- Interaction diagram for plastic resistance of section to  $N$  and  $M$
- Direct calculation of second-order effects of  $M$  and of initial bow



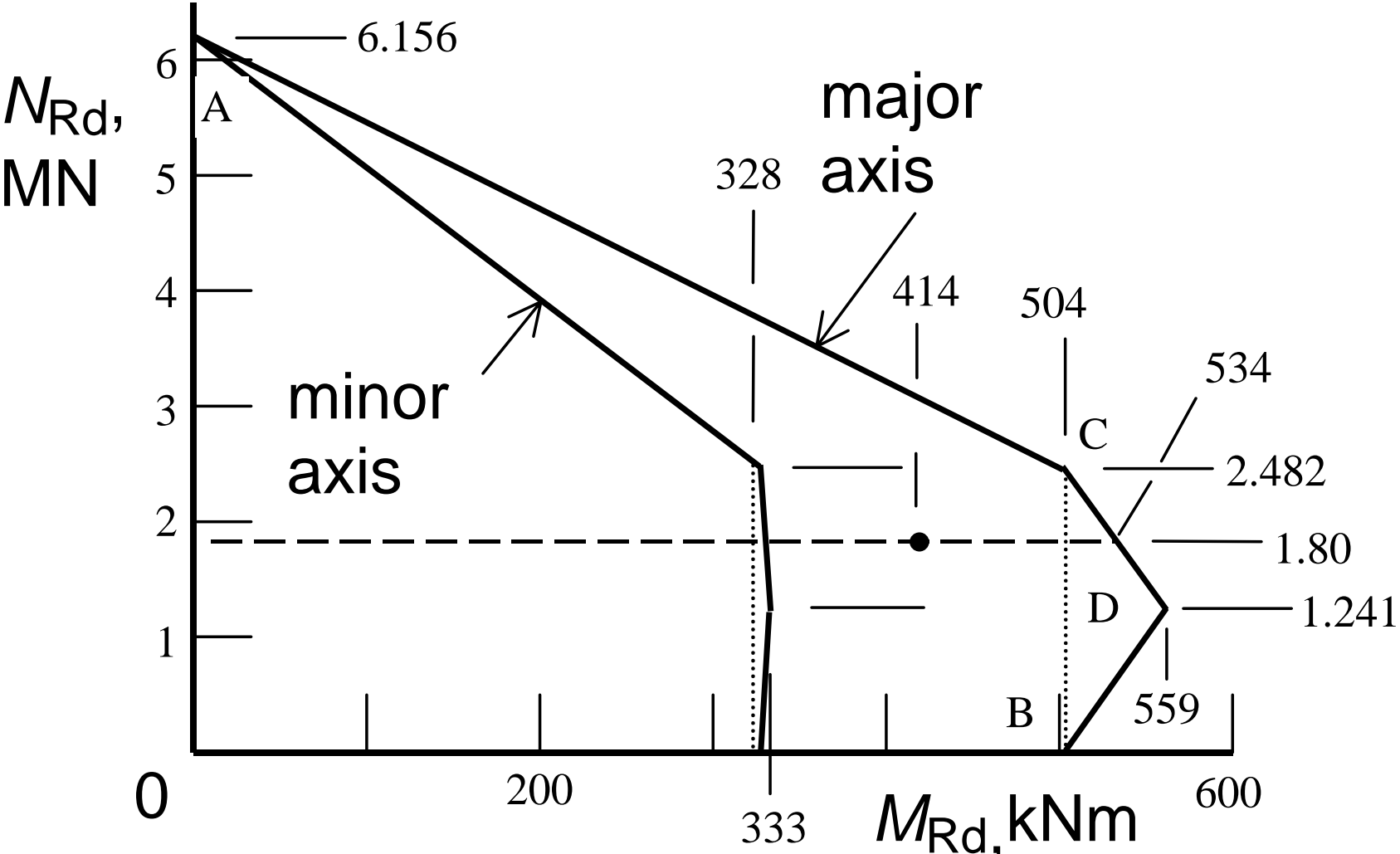
# Interaction curve and polygon for composite column



# First- and second-order bending moments in a column of length 7.0 m



# Interaction polygons for cross-section of a composite column



# How to start to use EN 1994-1-1 for a building structure

- Look at EN 1990, especially Annex A1
- Look at EN 1991-1, on actions for buildings
- Use EC2-1-1 and EC3-1-1 (and 3-1-8) for simple structures (and for bridges, EC3-1-5 and 3-1-9)
- Study worked examples on EC4-1-1
- In your copy of each code, enter changes from its NA

# NDPs from National Annexes

## For EC3-1-1:

- For plastic global analysis, higher properties for steel
- Different curves for lateral-torsional buckling
- Lower  $\gamma_{M2}$  for fracture resistance in tension

## For EC4-1-1:

- Minimum sheeting thickness: 'bare metal' not 'nominal'
- Different  $\gamma_V$  for studs in profiled sheeting